

**2013 Floodplain Management Conference**

September 3-6, Anaheim, California

# Creating a Safer Tomorrow

*Building Resilience through Integrated Flood Risk Management*





# Creating a Safer Tomorrow

## FLOODPLAIN MANAGEMENT CONFERENCE

September 3-6, 2013 • Anaheim, California

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Friday, September 6, 2013

Room/ Time	Workshop/ Event
7:30   8:30	CONTINENTAL BREAKFAST
8:00   9:00	<p>FORUM: What are Your National and State Floodplain Management Policy Priorities? Mapping, climate change, land development practices, stormwater regulations, building codes, levee certification, floodways, funding, etc.: What's on your mind? Join FEMA, ASFPM and NAFSMA in a fun, lively and interactive dialogue on your priorities for state and national policies in floodplain management.</p> <p>Moderator: Lovanka Todt, Executive Director, Floodplain Management Association</p> <ul style="list-style-type: none"> <li>• Pal Hegedus, PE, FEMA Past Chair, RBF Consulting—A Baker Company</li> <li>• Chad Berginnis, CFM, Executive Director, Association of State Floodplain Managers</li> <li>• Dusty Williams, PE, CFM, Chair, National Association of Flood and Stormwater Agencies &amp; General Manager/ Chief Engineer, Riverside County Flood Control &amp; Water Conservation District</li> </ul>
8:30   12:00	CFM Exam. Advance registration with ASFPM required at <a href="http://www.floods.org">www.floods.org</a> .

## CONCURRENT MORNING SESSIONS

9:00   10:30	<p>Integrated Approaches to Flood Management</p> <p>Moderator: David H. Pohl, PhD, PE, Senior Program Manager, ESA/PWA</p> <p>Panelists:</p> <ul style="list-style-type: none"> <li>• Eileen Takata, Watershed Program Manager, U.S. Army Corps of Engineers, Los Angeles District</li> <li>• Cid Tesoro, PE, Program Manager, County of San Diego Public Works</li> <li>• Terri Grant, PE, Assistant Division Engineer, Watershed Management Division, Los Angeles County Public Works</li> </ul>
9:00   10:30	<p>TOPICS IN 2D MODEL VALIDATION</p> <p>Session Chair: Andrew Trelease, PE, Principal Civil Engineer, Clark County Regional Flood Control District</p> <ul style="list-style-type: none"> <li>• 2D Model Calibration of 1998 B Nile Storm. Hassan Kasraie, Kasraie Consulting, Ventura, CA</li> <li>• The Effect of Orientation of Square 2D Grid Elements on Possible Hydraulic Bias in Multiple-Direction Flow Hydraulic Models. Neil M. Jordan, PE, DWRE, Hromadka &amp; Associates and Theodore Hromadka, Department of Mathematical Science, United States Military Academy, West Point, NY</li> <li>• Benchmarking SWMM5/PCSWMM 2D Model Performance. Rob James, Computational Hydraulics International</li> </ul>
9:00   10:30	SPECIAL AGENCY MEETING: Southern California Silver Jackets Kick-Off Summit
10:30   12:00	<p>2-D Modeling Roundtable: What have we learned? Moderator: Pal Hegedus, PE, FEMA Past Chair, RBF Baker. Join the 2D Modeling "Geeks" in this interactive forum to examine conference findings and develop recommendations for improving the use of 2D modeling in floodplain analysis.</p>
10:45   12:00	<p>Regulatory Challenges In Managing Flood Risk</p> <p>Moderator: David H. Pohl, PhD, PE, Senior Program Manager, ESA/PWA</p> <p>Panelists:</p> <ul style="list-style-type: none"> <li>• Greg Gearhart, Senior Water Resources Engineer, California State Water Resources Control Board</li> <li>• Corice Farrar, Chief, Orange and Riverside Counties Section, Regulatory Division, U.S. Army Corps of Engineers, Los Angeles District</li> <li>• Nardy Khan, PE, Engineering Supervisor, Projects and Regulatory/Permits Unit, Orange County Public Works</li> <li>• Jason Uhley, Chief of Watershed Protection Program, Riverside County Flood Control &amp; Water Conservation District</li> </ul>

**FLOODPLAIN MANAGEMENT ANNUAL CONFERENCE &  
2-D MODELING SYMPOSIUM  
September 3-6, Marriott Hotel, Anaheim, California**

September 3<sup>rd</sup> 2-D Modeling Symposium  
Exploring "Best Practices" for 2-D Flood Modeling and Mapping

•  
**Engineering Method Selection and Application**

**The Effect of Orientation of Square 2D Grid Elements on Possible Hydraulic Bias in  
Multiple-Direction Flow Hydraulic Models**

N. Jordan (1), Presenter; and T.V. Hromadka II (2)

**Abstract:**

Multiple directional flow models, such as occasionally used and developed for modeling of unsteady flow in two dimensions, are governed by the well-known flow equations of continuity and momentum. Some of these models use quasi one-dimensional flow rate estimates of flows as applied in the four principal directions along Cartesian coordinate axes and then budget the source and receiving modeling control volumes or cells in order to track mass balance or mass conservation. In this paper is presented the results of research into this topic that are presented in detail and published in the Journal of Hydrology. This paper concludes that mis-alignment of an array of square grid elements in a steady uniform flow results in the greatest error (slight underestimation of flow depth) when the misalignment angle is about 13 degrees, not 45 degrees as previously thought. The bias is about the same as or less than the magnitude of other uncertainties in hydraulic models, and would be compensated for during customary model calibration. Because of the near-uniformity of the slight bias from about 5 degrees to 85 degrees mis-alignment, it is concluded that the array of square elements can be applied over a complex drainage system without respect to topographic flow directions.

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**THE EFFECT OF ORIENTATION OF  
SQUARE 2D GRID ELEMENTS ON  
POSSIBLE HYDRAULIC BIAS IN  
MULTIPLE-DIRECTION FLOW  
HYDRAULIC MODELS**

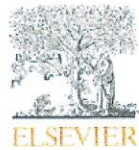
**N.M. JORDAN and T.V. HROMADKA**

**FLOODPLAIN MANAGEMENT  
ANNUAL CONFERENCE & 2-D  
MODELING SYMPOSIUM**

**SEPTEMBER 6, 2013**

# A REPORT OF FINDINGS FROM RESEARCH PUBLISHED IN THE JOURNAL OF HYDROLOGY

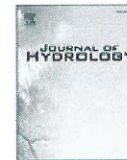
Journal of Hydrology 389 (2010) 177–185



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journal homepage: [www.elsevier.com/locate/jhydrol](http://www.elsevier.com/locate/jhydrol)



## Manning's equation and two-dimensional flow analogs

T.V. Hromadka II<sup>a,b,1</sup>, R.J. Whitley<sup>c,2</sup>, N. Jordan<sup>d,\*</sup>, T. Meyer<sup>a,3</sup>

<sup>a</sup> Department of Mathematical Sciences, United States Military Academy, West Point, NY, USA

<sup>b</sup> Professor Emeritus, California State University, CA, USA

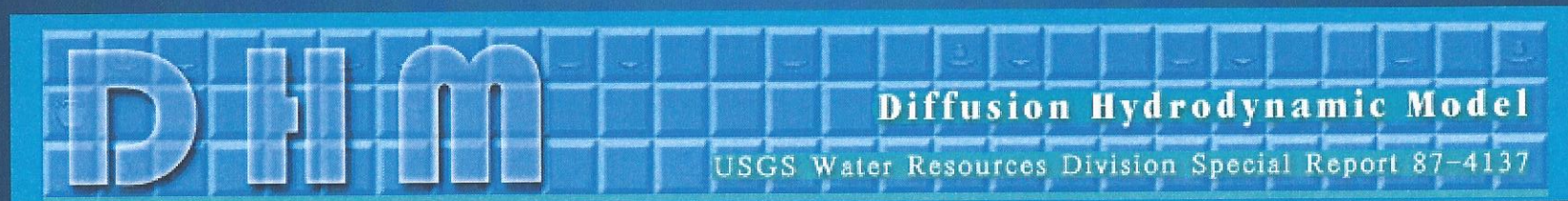
<sup>c</sup> Department of Mathematics, University of California, Irvine, CA, USA

<sup>d</sup> Exponent Failure Analysis, 320 Goddard Way, Ste 200, Irvine, CA 92618, USA

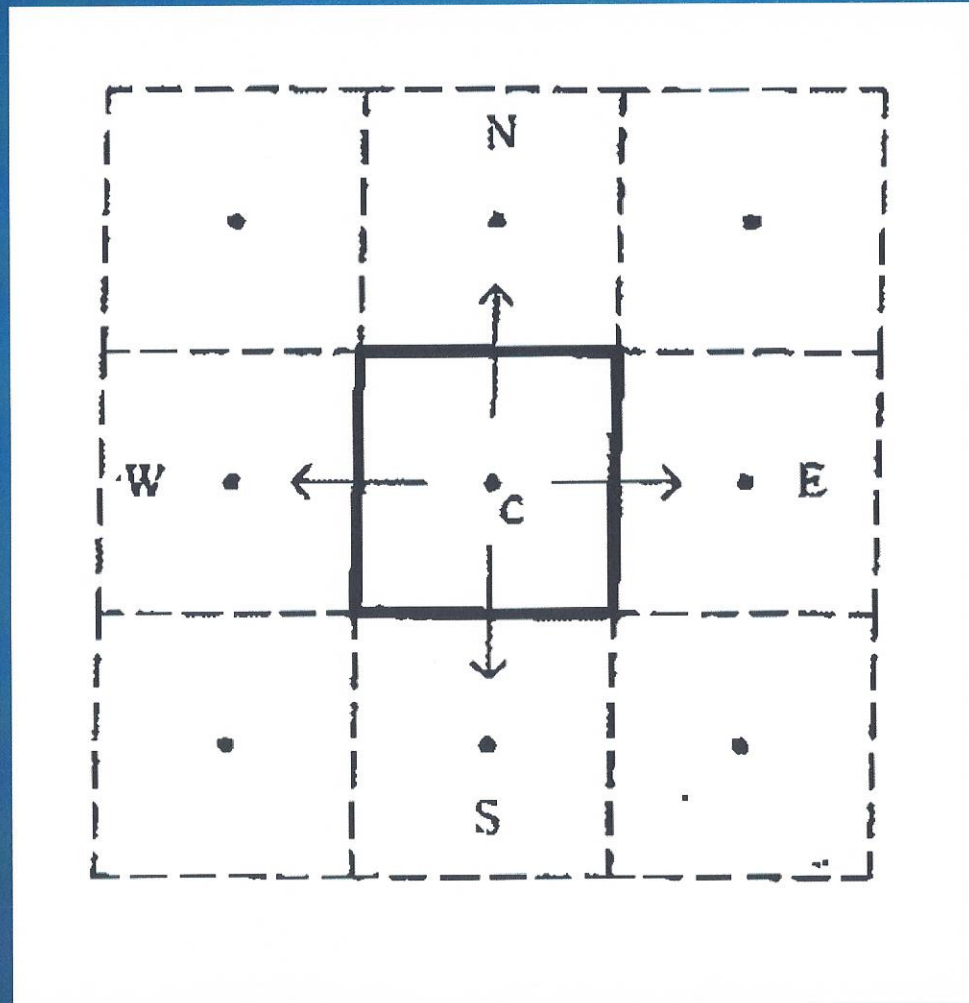
RESEARCH TO ANSWER SECOND OF TWO QUESTIONS  
RAISED IN APPLICATION OF US GEOLOGICAL SURVEY 2-D  
MODEL DEVELOPED BY HROMADKA & YEN IN 1987,  
WHICH USED MANUAL GRIDDING OF SQUARE GRID  
ELEMENTS OVER TOPOGRAPHIC SURFACE, ALIGNED  
APPROXIMATELY TO FLOW STREAMLINES.

**WHAT IF SQUARE GRID ELEMENTS ARE NOT  
ALIGNED WITH FLOW STREAMLINES?**

PAPER REPRINT AND SUPPORT DOCUMENTS AVAILABLE AT  
<http://www.diffusionhydrodynamicmodel.com/index2.html>

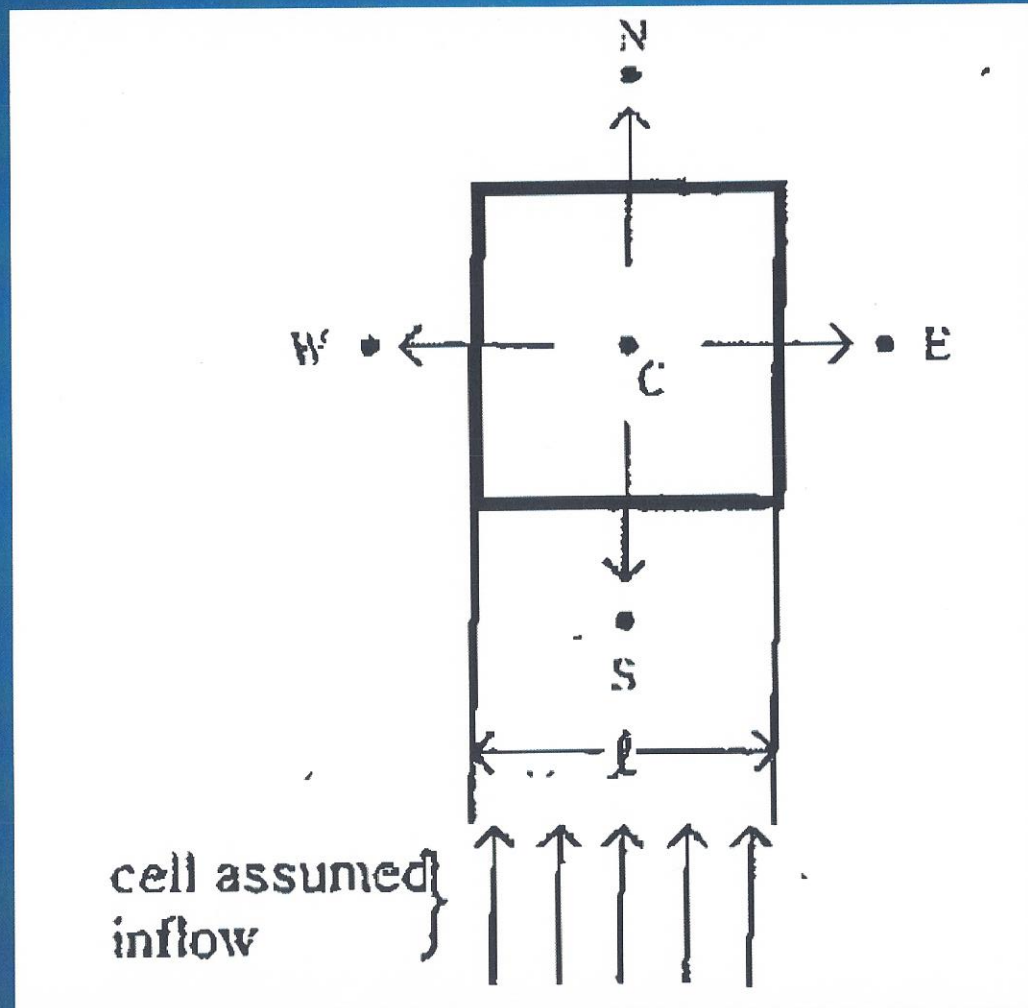


# USGS DIFFUSION HYDRODYNAMIC MODEL TILES A TOPOGRAPHIC SURFACE WITH SQUARE GRID ELEMENTS





INTENT IN 1987 WAS TO ALIGN THE GRID ELEMENTS WITH FLOW STREAMLINES



# HROMADKA & YEN (1987) USED TILING ALIGNED WITH COMPASS DIRECTION . . .

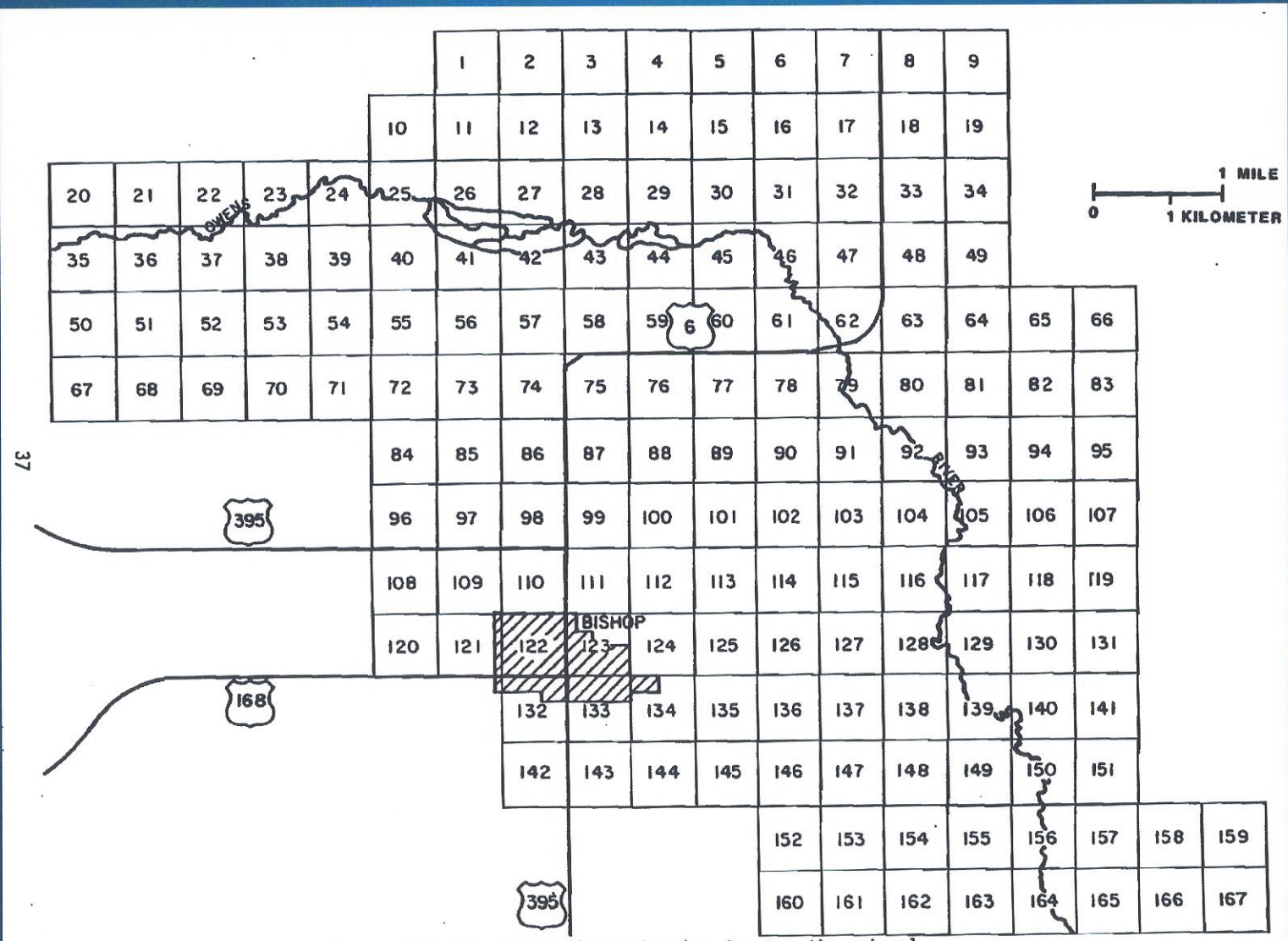
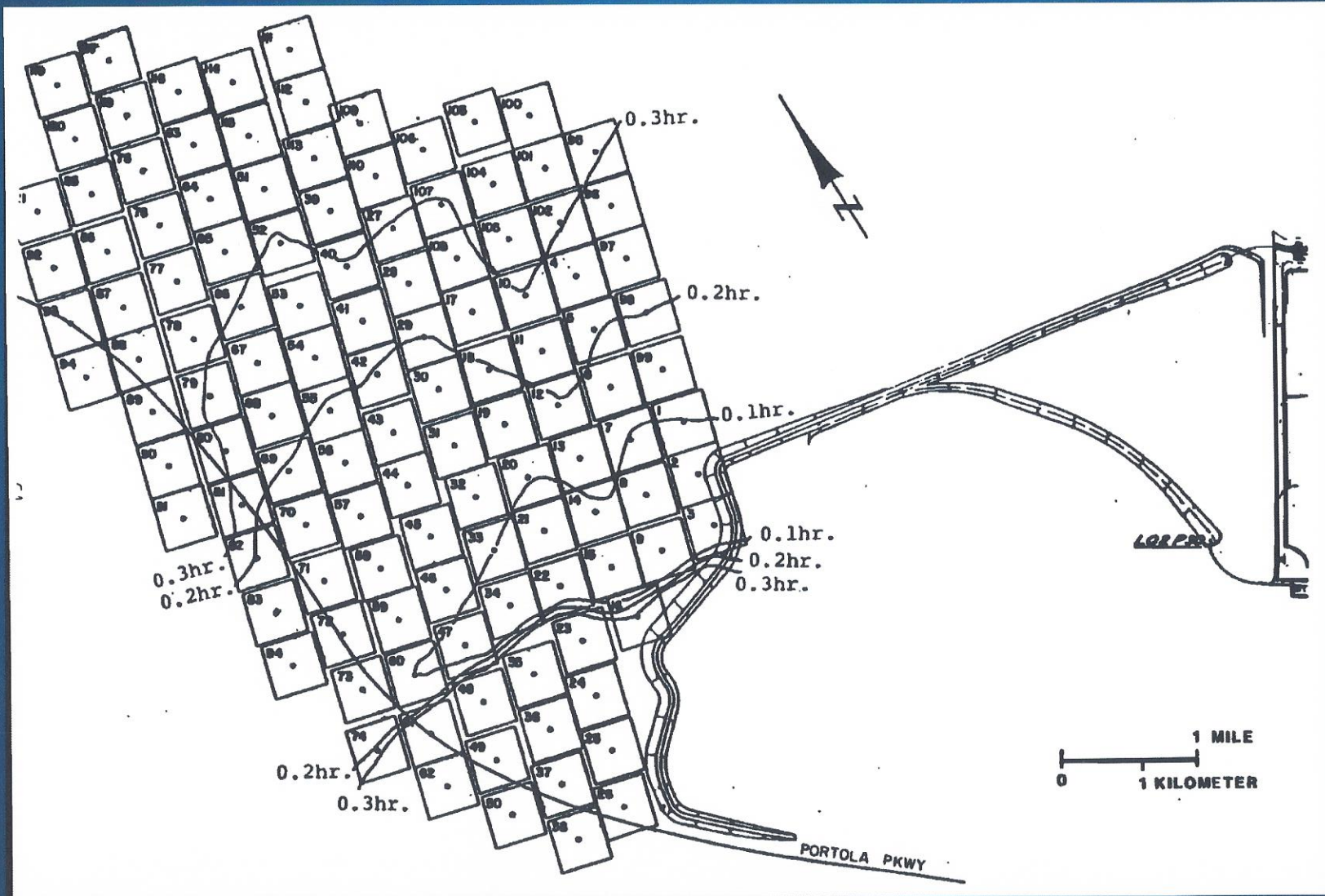


Figure 10.--Floodplain discretization for two-dimensional diffusion hydrodynamic model.

AND ALSO WITH APPROXIMATE FLOW STREAMLINES.



SQUARES ROTATED AND APPROXIMATELY OVERLAPPED

# WORST CASE WAS ORIGINALLY CONSIDERED TO BE 45 DEGREES OUT OF ALIGNMENT . . .

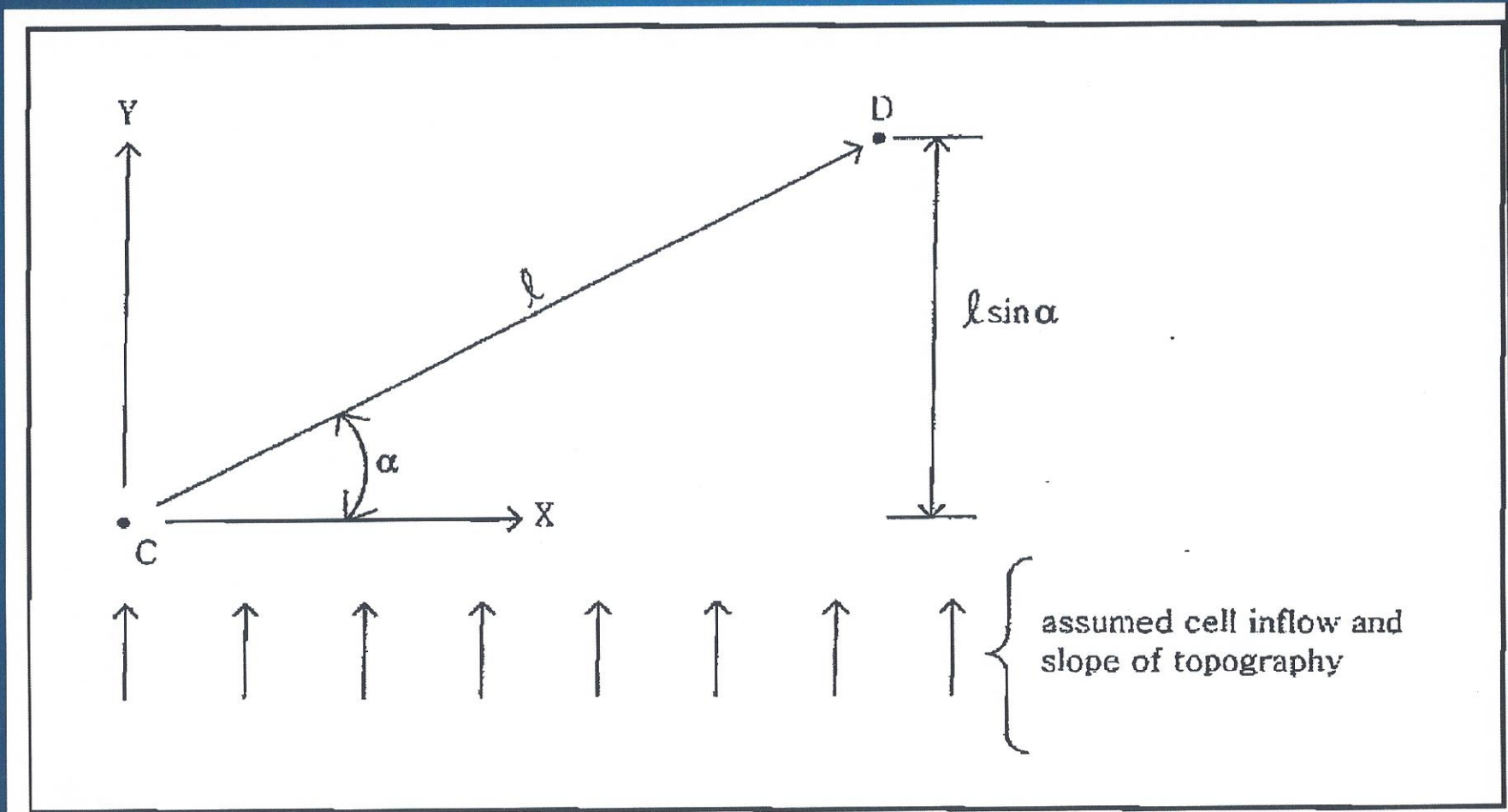


Figure 7. Calculations of Topographic Slope Along Flow Vector Between Nodes C and D.

**BUT . . .**

## MIS-ALIGNMENT EFFECT NOTED BY HORRITT & BATES (2001)

HYDROLOGICAL PROCESSES

*Hydrol. Process.* **15**, 825–842 (2001)

DOI: 10.1002/hyp.188

**Predicting floodplain inundation: raster-based modelling versus the finite-element approach**

M. S. Horritt\* and P. D. Bates

*School of Geographical Sciences, University of Bristol, University Road, Bristol BS8 1SS, UK*

Abstract:

“A possible criticism of the cellular approach is that it fails to reproduce some (intuitively correct) features of floodplain flows. **For example, flow may not be parallel to the free surface gradient, depending on the orientation of the free surface slope and the model grid**, but the two approaches agree when the slope is parallel to one of the grid axes. The differences between the two models can be derived analytically for a free surface slope of unit magnitude as a function of direction. **The flow vectors differ in magnitude by a mean value of c. 20% and in angle by c. 10°**. Although these deviations may be compensated for partially in the friction calibration process (the cellular approximation predicts larger flows than the diffusive approximation), the effect on the bulk flow behaviour of the model is unclear, and so both approximations are tested in this study.”

**MAXIMUM EFFECT WAS AT ABOUT 10 DEGREES MISALIGNMENT.**

# ALIGNMENT QUANDARY WAS APPROACHED ANALYTICALLY AND EMPIRICALLY FOR A SQUARE GRID ELEMENT IN STEADY UNIFORM FLOW FIELD.

## THE ANALYTICAL APPROACH , STARTING WITH:

“For the considered four-direction flow analog, flow directions are in the x, y directions only, whereas in an unaligned flow, streamlines are at an angle  $h$  with the positive x-axis. For 2D grid size  $W$ , flow velocities in the projected x- and y-directions are obtained from the streamline flow velocity,  $v_s$ , by. . .[Eq. 11]”

$$\left. \begin{aligned} v_y &= v_s \sin\theta \\ v_x &= v_s \cos\theta \\ v_s^2 &= v_x^2 + v_y^2 \end{aligned} \right\} \quad (11)$$

**THE ANALYTICAL APPROACH , SOLUTION FOR THE TERM BETA, THE RATIO OF STEADY UNIFORM FLOW NORMAL DEPTH TO COMPUTED DEPTH:**

“Therefore, the factor, beta, for any angle, can be expressed as a ratio of normal depth to computed depth [Eq. 31].”

$$\beta(\theta) = y_n / h_4 \quad (31)$$

“for theta values between 0 and pi/2. Because qx and qy are known by the flow analog application, Eq. (31) is readily applied.”

**INTRODUCTION OF MANNING'S N SOLUTION FOR THE TERM GAMMA, THE FACTOR APPLIED TO MANNING'S N SO THAT COMPUTED DEPTH EQUALS NORMAL DEPTH FOR A SQUARE GRID ELEMENT IN STEADY UNIFORM FLOW FIELD.**

“3. Extension of Manning's equation

“From the previous section, use of a similar application of Manning's equation to flow vectors that are not in alignment with the considered SSUF problem streamlines may introduce a bias in the estimation of hydraulic properties. In this section, the identified possible bias is addressed by redefining the application of the flow vector friction factor. For the considered SSUF problem, equating inflow into the grid to grid outflow by the four-direction flow analog gives [Eq. 32],

$$\frac{1}{n} y_n^{5/3} s_o^{1/2} W(\cos \theta + \sin \theta) = \frac{1}{\gamma n} y_n^{5/3} W(s_{oy}^{1/2} + s_{ox}^{1/2})$$

“where gamma is a factor applied to Manning's n value as applied in the four-direction flow analog such that  $h_{4-way} = y_n$  [normal depth].”



## EQUATION 33, GAMMA AS A FUNCTION OF ANGLE THETA

“From Eq. (16) and combining with Eq. (32) gives gamma as a function of angle theta [Eq. 33]:”

$$\gamma(\theta) = \frac{\sqrt{\cos \theta} + \sqrt{\sin \theta}}{\cos \theta + \sin \theta}$$

# GAMMA AS A FUNCTION OF ANGLE THETA

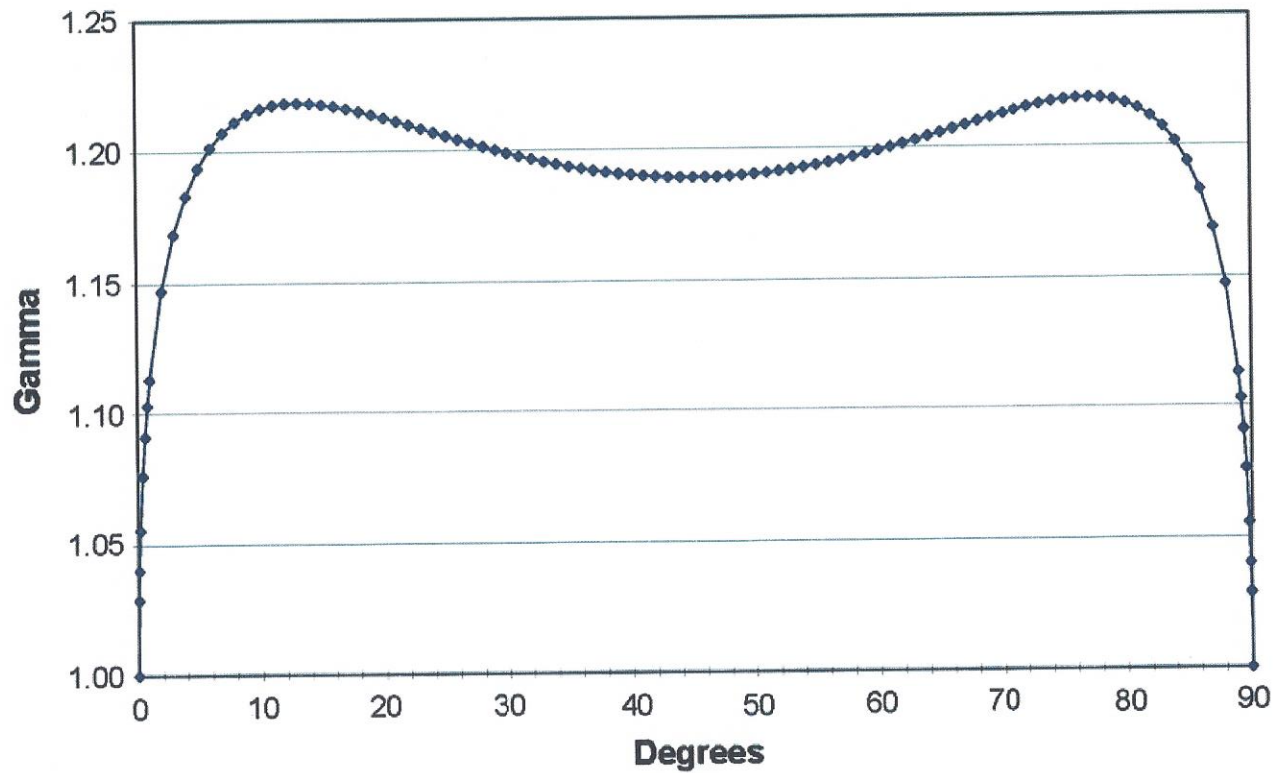
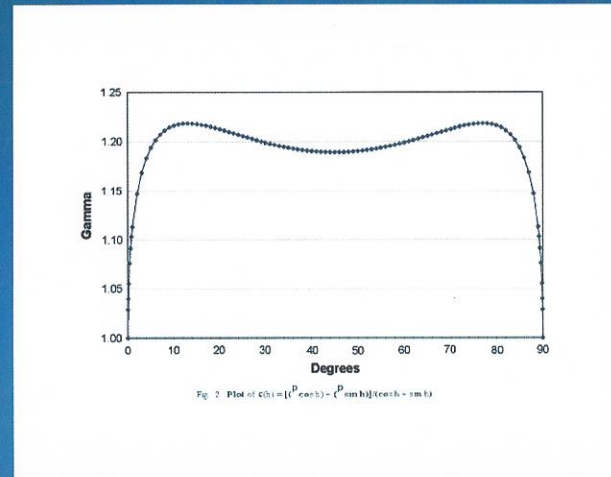


Fig. 2. Plot of  $\mathfrak{C}(\mathfrak{h}) = [(\mathfrak{C}^{\mathfrak{p}} \cosh \mathfrak{h}) + (\mathfrak{C}^{\mathfrak{p}} \sin \mathfrak{h})] / (\cosh \mathfrak{h} + \sin \mathfrak{h})$ .

## SUMMARY:



- MIS-ALIGNMENT ANGLE THAT RESULTS IN THE HIGHEST VALUE OF GAMMA (1.22) IS ABOUT 13 DEGREES.
- AVERAGE VALUE OF GAMMA TAKEN AT 1 DEGREE INCREMENTS FROM 0 TO 90 DEGREES IS 1.19.
- AVERAGE VALUE OF GAMMA TAKEN AT 1 DEGREE INCREMENTS FROM 5 TO 85 DEGREES IS 1.20.
- VALUE OF GAMMA AT 45 DEG IS EXACTLY  $2^{(1/4)}$  OR 1.19.

## **NOW THE HARD PART –**

**SET UP ARRAYS OF SQUARE GRID ELEMENTS AT INCREMENTAL ANGLES TO TEST THE EFFECT OF MISALIGNMENT.**

- **USGS DIFFUSION HYDRODYNAMIC MODEL.**
- **USE ARRAY OF 20 X 20 GRID ELEMENTS (400 TOTAL).**
- **USE SLOPE AND ROUGHNESS TO CREATE SHALLOW STEADY UNIFORM SUBCRITICAL FLOW 1 FT DEEP. SUBCRITICAL DRAWDOWN M2 CURVE.**
- **ROTATE ARRAY IN INCREMENTS OF 1:10 (6 deg), 1:5 (11 deg), 1:4 (14 deg), 1:3 (18 deg), 1:2 (27 deg) AND 1:1 (45 deg).**
- **NOTE THAT COMPUTED FLOW DEPTHS IN ALL ROTATED (MIS-ALIGNED) MODELS WERE SLIGHTLY LESS THAN ACTUAL NORMAL DEPTH.**
- **INCREMENTALLY INCREASE MANNING'S N UNTIL COMPUTED FLOW DEPTHS EQUALED ACTUAL NORMAL DEPTH.**

# ALIGNED MODEL

### DHM GEOMETRY FILE GENERATOR FOR WIDE RECTANGULAR CHANNEL CHECK

00 DHM FILE GEN.XLS

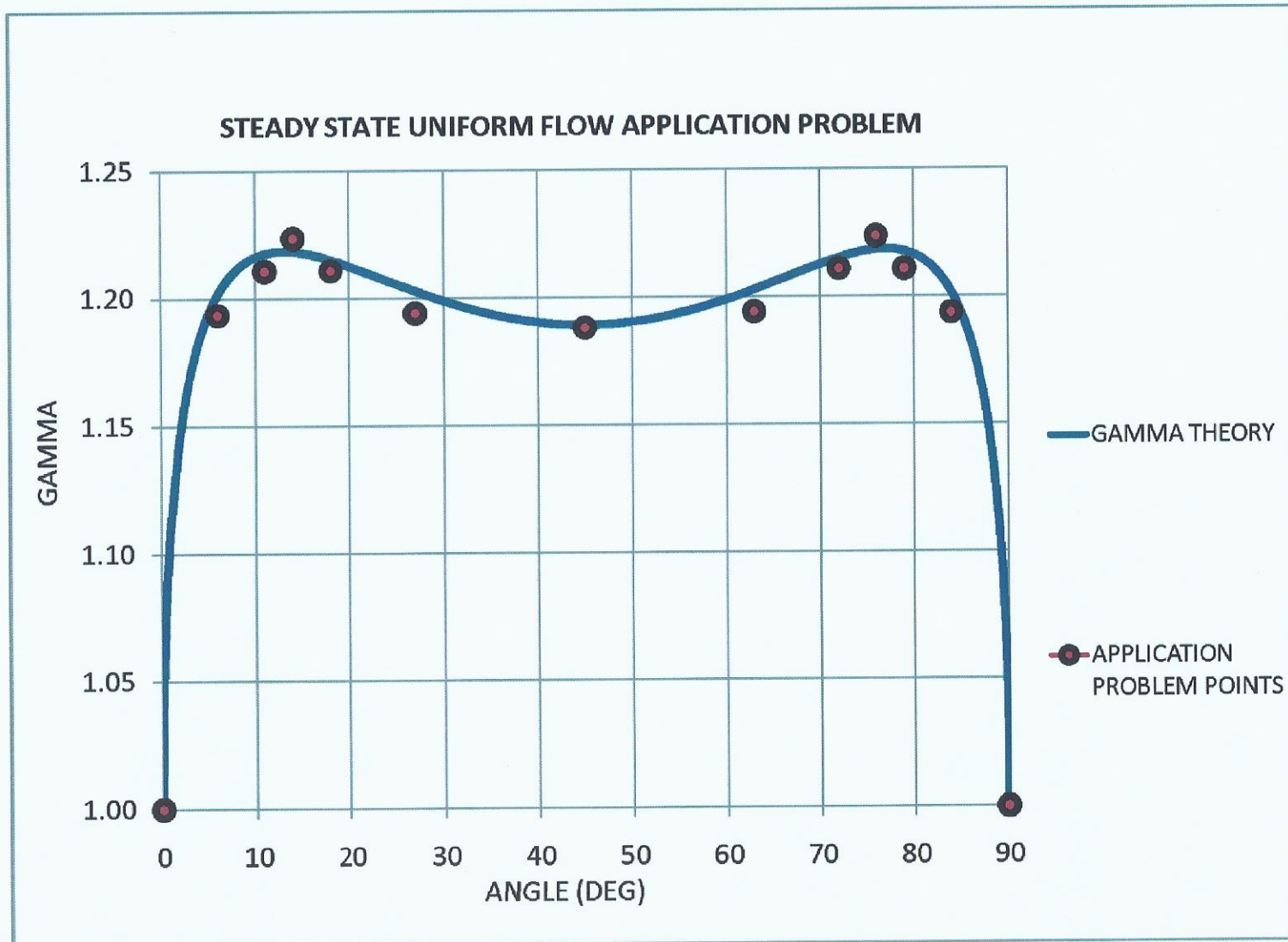
NOTE: TOPO LINE, CHANNEL CENTERLINE, SIDE LINE, CHANNEL RIGHT OF CUT OR RIVER OF BED, CHANNEL LEFT BANK PERPENDICULAR TO FLOWFIELD FORMULAS TO FOLLOW.

KEY	R	GRND NO	ELV.	MIN CH W	BEST	Centre	MIN CH H	CH TOP	CH BOT	CH TOP	CH BOT	TOTAL	CH TOP	CH BOT
	2	4	6	8	10	12	14	16	18	20	22	24	26	28
	30	32	34	36	38	40	42	44	46	48	50	52	54	56
	60	62	64	66	68	70	72	74	76	78	80	82	84	86
	90	92	94	96	98	100	102	104	106	108	110	112	114	116
	120	122	124	126	128	130	132	134	136	138	140	142	144	146
	150	152	154	156	158	160	162	164	166	168	170	172	174	176
	180	182	184	186	188	190	192	194	196	198	200	202	204	206
	210	212	214	216	218	220	222	224	226	228	230	232	234	236
	240	242	244	246	248	250	252	254	256	258	260	262	264	266
	270	272	274	276	278	280	282	284	286	288	290	292	294	296
	300	302	304	306	308	310	312	314	316	318	320	322	324	326
	330	332	334	336	338	340	342	344	346	348	350	352	354	356
	360	362	364	366	368	370	372	374	376	378	380	382	384	386
	390	392	394	396	398	400	402	404	406	408	410	412	414	416
	420	422	424	426	428	430	432	434	436	438	440	442	444	446
	450	452	454	456	458	460	462	464	466	468	470	472	474	476
	480	482	484	486	488	490	492	494	496	498	500	502	504	506
	510	512	514	516	518	520	522	524	526	528	530	532	534	536
	540	542	544	546	548	550	552	554	556	558	560	562	564	566
	570	572	574	576	578	580	582	584	586	588	590	592	594	596
	600	602	604	606	608	610	612	614	616	618	620	622	624	626
	630	632	634	636	638	640	642	644	646	648	650	652	654	656
	660	662	664	666	668	670	672	674	676	678	680	682	684	686
	690	692	694	696	698	700	702	704	706	708	710	712	714	716
	720	722	724	726	728	730	732	734	736	738	740	742	744	746
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	780	782	784	786	788	790	792	794	796	798	800	802	804	806
	810	812	814	816	818	820	822	824	826	828	830	832	834	836
	840	842	844	846	848	850	852	854	856	858	860	862	864	866
	870	872	874	876	878	880	882	884	886	888	890	892	894	896
	900	902	904	906	908	910	912	914	916	918	920	922	924	926
	930	932	934	936	938	940	942	944	946	948	950	952	954	956
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	990	992	994	996	998	1000	1002	1004	1006	1008	1010	1012	1014	1016
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	1140	1142	1144	1146	1148	1150	1152	1154	1156	1158	1160	1162	1164	1166
	1170	1172	1174	1176	1178	1180	1182	1184	1186	1188	1190	1192	1194	1196
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	1260	1262	1264	1266	1268	1270	1272	1274	1276	1278	1280	1282	1284	1286
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	1440	1442	1444	1446	1448	1450	1452	1454	1456	1458	1460	1462	1464	1466
	1470	1472	1474	1476	1478	1480	1482	1484	1486	1488	1490	1492	1494	1496
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	1560	1562	1564	1566	1568	1570	1572	1574	1576	1578	1580	1582	1584	1586
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	1650	1652	1654	1656	1658	1660	1662	1664	1666	1668	1670	1672	1674	1676
	1680	1682	1684	1686	1688	1690	1692	1694	1696	1698	1700	1702	1704	1706
	1710	1712	1714	1716	1718	1720	1722	1724	1726	1728	1730	1732	1734	1736
	1740	1742	1744	1746	1748	1750	1752	1754	1756	1758	1760	1762	1764	1766
	1770	1772	1774	1776	1778	1780	1782	1784	1786	1788	1790	1792	1794	1796
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	1950	1952	1954	1956	1958	1960	1962	1964	1966	1968	1970	1972	1974	1976
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	2070	2072	2074	2076	2078	2080	2082	2084	2086	2088	2090	2092	2094	2096
	2100	2102	2104	2106	2108	2110	2112	2114	2116	2118	2120	2122	2124	2126
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	2160	2162	2164	2166	2168	2170	2172	2174	2176	2178	2180	2182	2184	2186
	2190	2192	2194	2196	2198	2200	2202	2204	2206	2208	2210	2212	2214	2216
	2220	2222	2224	2226	2228	2230	2232	2234	2236	2238	2240	2242	2244	2246
	2250	2252	2254	2256	2258	2260	2262	2264	2266	2268	2270	2272	2274	2276
	2280	2282	2284	2286	2288	2290	2292	2294	2296	2298	2300	2302	2304	2306
	2310	2312	2314	2316	2318	2320	2322	2324	2326	2328	2330	2332	2334	2336
	2340	2342	2344	2346	2348	2350	2352	2354	2356	2358	2360	2362	2364	2366
	2370	2372	2374	2376	2378	2380	2382	2384	2386	2388	2390	2392	2394	2396
	2400	2402	2404	2406	2408	2410	2412	2414	2416	2418	2420	2422	2424	2426
	2430	2432	2434	2436	2438	2440	2442	2444	2446	2448	2450	2452	2454	2456
	2460	2462	2464	2466	2468									





# APPLICATION PROBLEM MODEL RESULTS





## APPLICATION PROBLEM MODEL RESULTS –

- SEVEN TEST MISALIGNMENTS (AND SIX MORE BY SYMMETRY) AGREE WITH THEORETICAL VALUES OF GAMMA.
- ZERO DEGREE (AND 90 DEGREE) ALIGNMENT, AND 45 DEGREE MISALIGNMENT AGREE EXACTLY WITH THEORETICAL VALUES.

## IS THIS BIAS A PROBLEM?

*“Investigation of a ratio with respect to Manning’s  $n$ , as opposed to introducing a new factor into Manning’s equation, is justified for the typical application of USGS DHM to analyze shallow overland flow in floodplains. Engman (1989) has shown that the governing flow equations can be solved with proper boundary conditions and the selection of only one parameter, Manning’s  $n$ .”*

## IS THIS BIAS A PROBLEM?

*“It might be concluded that Manning’s  $n$  could be adjusted for each element so that computed depths match actual depths. However, the small variation in Manning’s  $n$  across the wide range of streamline flow angles with respect to the element alignments makes this an ineffective process that might indeed be superfluous. For usual cases where random streamline trajectory variability within the floodplain flow is greater than a few degrees from perfect alignment, the ratio  $\gamma(\Theta)$  appears to be implicitly included in the Manning’s  $n$  values.”*

**“IT CAN BE CONCLUDED THAT THE ARRAY OF SQUARE ELEMENTS MAY BE APPLIED OVER THE DIGITAL TERRAIN MODEL WITHOUT RESPECT TO TOPOGRAPHIC FLOW DIRECTIONS.”**

**ANOTHER TEST –  
FOR ANOTHER DAY.**

**ROTATE AN ARRAY  
OVER A GAGED  
WATERSHED WITH  
VERY COMPLEX  
TOPOGRAPHY.**

**IS THERE ANY EFFECT  
ON USGS DHM  
RAINFALL-RUNOFF  
CALCULATION?**

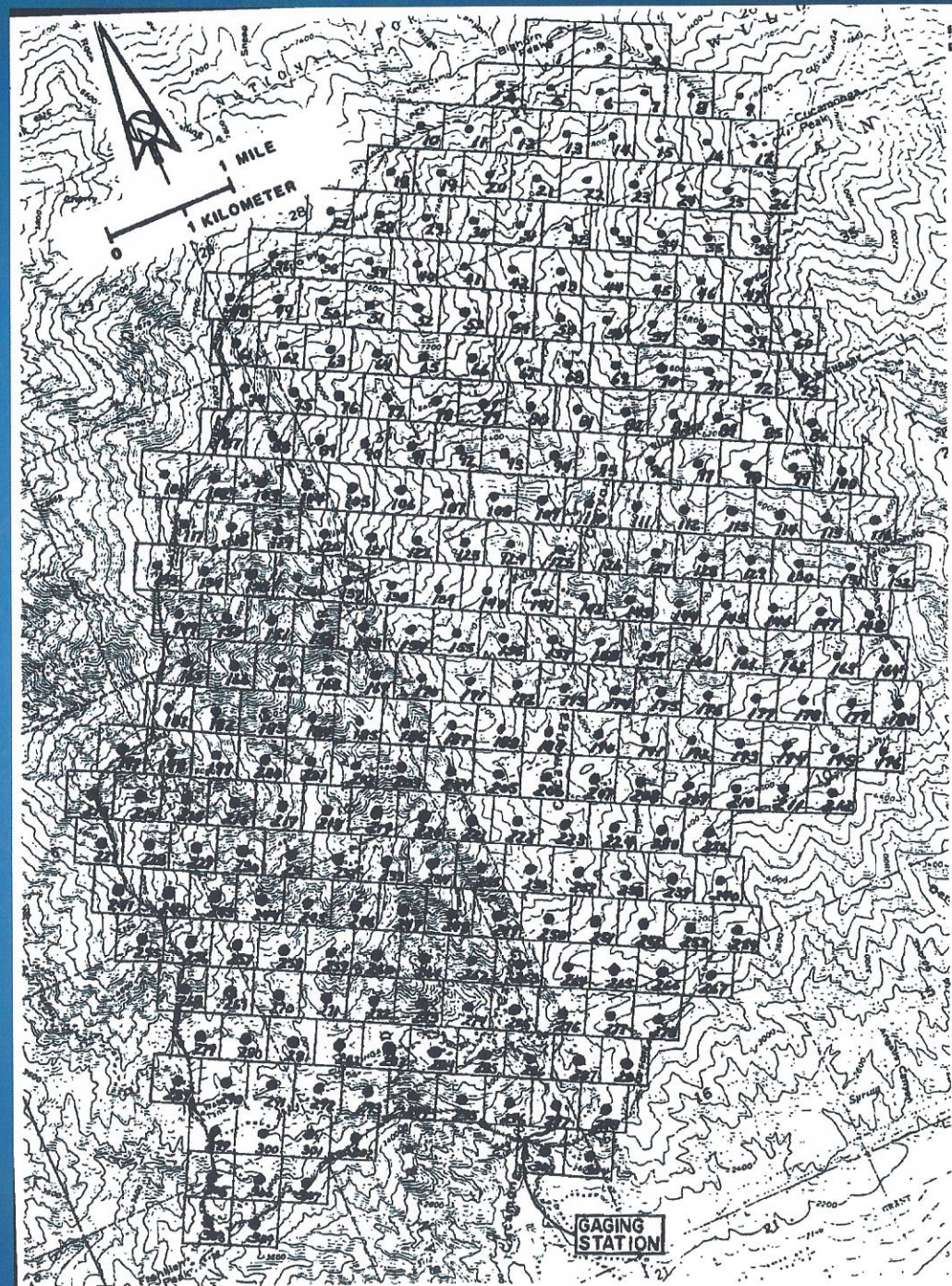
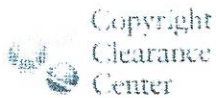
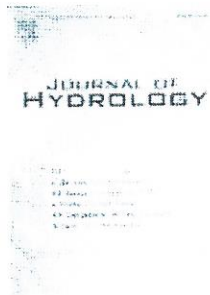


Figure 17.--Cucamonga Creek Discretization

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**Title:** Manning's equation and two-dimensional flow analogs  
**Author:** T.V. Hromadka,R.J. Whitley,N. Jordan,T. Meyer  
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